



## FORCED VIBRATIONS OF SIMPLY SUPPORTED ANISOTROPIC RECTANGULAR PLATES

P. A. A. LAURA

*Institute of Applied Mechanics (CONICET) and Department of Engineering,  
Universidad Nacional del Sur, 8000 Bahía Blanca, Argentina*

AND

D. R. AVALOS AND H. A. LARRONDO

*Physics Department, School of Engineering,  
Universidad Nacional de Mar Del Plata, 7600 Mar del Plata, Argentina*

(Received 12 August 1998)

### 1. INTRODUCTION

There is a limited amount of information on forced vibrations of isotropic plates or slabs [1–4]. The situation is more critical in the case of anisotropic plates [5].

The present study deals with the determination of the transverse, dynamic response of simply supported, rectangular plates of generalized anisotropy, subjected to a uniformly distributed  $p_0 \cos \omega t$ -type excitation.

The governing functional is formulated using Lekhnitskii's approach and notation [5] and the problem is solved using the Rayleigh–Ritz method by means of a double Fourier series which constitutes the exact solution of the mathematical system when the plate is isotropic or orthotropic.\*

### 2. APPROXIMATE SOLUTION

In the case of forced vibrations the problem under investigation is governed by the functional.

$$J = \frac{1}{2} \int_{A_p} \left\{ D_{11} \left( \frac{\partial^2 W'}{\partial x'^2} \right)^2 + 2D_{12} \frac{\partial^2 W'}{\partial x'^2} \frac{\partial^2 W'}{\partial y'^2} + D_{22} \left( \frac{\partial^2 W'}{\partial y'^2} \right)^2 + 4 \left( \frac{\partial^2 W'}{\partial x' \partial y'} \right)^2 \right. \\ \left. \times \left[ D_{16} \left( \frac{\partial^2 W'}{\partial x'^2} \right) + D_{26} \left( \frac{\partial^2 W'}{\partial y'^2} \right) \right] + 4D_{66} \left( \frac{\partial^2 W'}{\partial x' \partial y'} \right)^2 \right\} dx' dy' \\ - \frac{\rho h \omega^2}{2} \int_{A_p} W'^2 dx' dy' - p_0 \int_{A_p} W' dx' dy', \quad (1)$$

\* Obviously: when the orthotropic material characteristics are parallel to the plate edges.

where  $W'$  is the true displacement amplitude of the plate and  $A_p$  is the total area of the plate plan form. In equation (1), the  $D_{ij}$  are the well known flexural rigidities of the (anisotropic) plate, which, for an isotropic plate, take the simplest form

$$D_{11} = D_{22} = \frac{Eh^3}{12(1-\nu^2)}, \quad D_{12} = \nu D_{11}, \quad D_{66} = \frac{(1-\nu)}{2} D_{11}, \quad D_{16} = D_{26} = 0, \quad (2)$$

If the length of the sides of the rectangular plate are  $a$  and  $b$  in the  $x$  and  $y$  directions, respectively, equation (1) can be cast in a non-dimensional form by introducing

$$W = W'/a, \quad x = x'/a, \quad y = y'/b. \quad (3)$$

One gets, for the functional for the whole system of Figure 1

$$\begin{aligned} J_{nd} = \frac{2J}{rD_{11}} = \int_{A_p} & \left[ \left( \frac{\partial^2 W}{\partial x^2} \right)^2 + \frac{2}{r^2} \frac{D_{12}}{D_{11}} \frac{\partial^2 W}{\partial x^2} \frac{\partial^2 W}{\partial y^2} + \frac{1}{r^4} \frac{D_{22}}{D_{11}} \left( \frac{\partial^2 W}{\partial y^2} \right)^2 + \frac{4}{r} \left( \frac{\partial^2 W}{\partial x \partial y} \right) \right. \\ & \left. + \left( \frac{D_{16}}{D_{11}} \frac{\partial^2 W}{\partial x^2} + \frac{1}{r^2} \frac{D_{26}}{D_{11}} \frac{\partial^2 W}{\partial y^2} \right) + \frac{4}{r^2} \frac{D_{26}}{D_{11}} \left( \frac{\partial^2 W}{\partial x \partial y} \right)^2 \right] dx dy \\ & - \Omega^2 \int_{A_p} W^2 dx dy - 2P_0 \int_{A_p} W dx dy, \end{aligned} \quad (4)$$

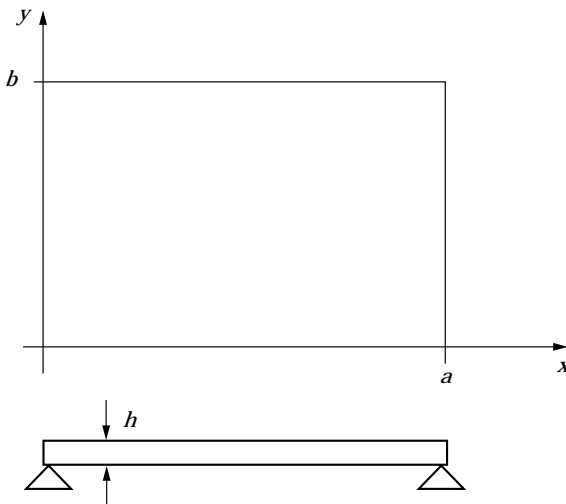


Figure 1. Vibrating system under consideration.

where, as usual,

$$\Omega^2 = \frac{\rho h \omega^2 a^4}{D_{11}} \quad (5)$$

is the non-dimensional frequency coefficient;

$$P_0 = \frac{p_0 a^3}{D_{11}} \quad (6)$$

is the non-dimensional load and

$$r = \frac{b}{a} \quad (7)$$

is the aspect ratio of the plate.

The plate response is now expressed in the classical (non-dimensional) form by means of a double Fourier series:

$$W_a(x, y) = \sum_{n=1}^N \sum_{m=1}^M b_{mn} \sin(m\pi x) \sin(n\pi y). \quad (8)$$

One gets as a final expression for the functional the sum

$$\begin{aligned} J_{nd} = & \sum_{n=1}^N \sum_{m=1}^M \sum_{l=1}^N \sum_{k=1}^M b_{kl} b_{mn} \left\{ \pi^4 \left[ m^2 k^2 + 2 \frac{D_{12}}{D_{11}} \frac{k^2 n^2}{r^2} + \frac{D_{22}}{D_{11}} \frac{n^2 l^2}{r^4} \right] A_{SS} \right. \\ & - 4\pi^4 \left[ \frac{k^2 mn}{r} \frac{D_{16}}{D_{11}} + \frac{l^2 mn}{r^3} \frac{D_{26}}{D_{11}} \right] A_{SC} + \pi^4 \left[ \frac{4D_{66}}{D_{11}} \frac{klmn}{r^2} \right] A_{CC} - \Omega^2 A_{SS} \left. \right\} \\ & - 2P_0 \frac{4}{\pi^2} \sum_{n=0}^{(N-1)/2} \sum_{m=0}^{(M-1)/2} b_{(2n+1)(2m+1)} / [(2m+1)(2n+1)], \quad (9) \end{aligned}$$

where

$$A_{SS} = \int_{A_p} \sin(k\pi x) \sin(l\pi y) \sin(m\pi x) \sin(n\pi y) dx dy, \quad (10)$$

$$A_{SC} = \int_{A_p} \sin(k\pi x) \cos(m\pi x) \sin(l\pi y) \cos(n\pi y) dx dy, \quad (11)$$

$$A_{CC} = \int_{A_p} \cos(k\pi x) \cos(l\pi y) \cos(m\pi x) \cos(n\pi y) dx dy, \quad (12)$$

and (as shown) the last double sum in equation (9) must be taken only for odd values of the sub-indexes of  $b_{kl}$ .

When dealing with the situation of forced vibrations, using the minimization condition in expression (9),

$$\frac{\partial J}{\partial b_{mn}} = 0, \quad m, n = 1, \dots, M, N, \quad (13)$$

one gets a linear system of non-homogeneous equations in the  $b_{mn}$ 's.

Once these are determined one is able to calculate approximate values of the displacement amplitude

$$W_a(x, y) \left/ \left( \frac{p_0 a^4}{D_{11}} \right) \right. = \sum_{n=1}^N \sum_{m=1}^M \frac{b_{mn}}{P_0} \sin(m\pi x) \sin(n\pi y). \quad (14)$$

and of the stress resultants. In the case of bending moments, their amplitudes are given by the expressions

$$\begin{aligned} \frac{M_x}{p_0 a^2} = & \sum_{n=1}^N \sum_{m=1}^M \frac{b_{mn}}{P_0} \left\{ \sin(m\pi x) \sin(n\pi y) \left[ m^2 \pi^2 + \frac{D_{12}}{D_{11}} \frac{n^2 \pi^2}{r^2} \right] \right. \\ & \left. - \cos(m\pi x) \cos(n\pi y) \left[ 2 \frac{D_{16}}{D_{11}} \frac{mn\pi^2}{r} \right] \right\}, \end{aligned} \quad (15)$$

$$\begin{aligned} \frac{M_y}{p_0 a^2} = & \sum_{n=1}^N \sum_{m=1}^M \frac{b_{mn}}{P_0} \left\{ \sin(m\pi x) \sin(n\pi y) \left[ \frac{D_{12}}{D_{11}} m^2 \pi^2 + \frac{D_{22}}{D_{11}} \frac{n^2 \pi^2}{r^2} \right] \right. \\ & \left. - \cos(m\pi x) \cos(n\pi y) \left[ 2 \frac{D_{26}}{D_{11}} \frac{mn\pi^2}{r} \right] \right\}. \end{aligned} \quad (16)$$

### 3. NUMERICAL RESULTS

All calculations were performed taking  $D_{12}/D_{11} = 0.3$ ;  $D_{22}/D_{11} = 0.5 = D_{66}/D_{11}$ ; and  $D_{16}/D_{11} = 1/3 = D_{26}/D_{11}$ .

TABLE 1

*Non-dimensional displacement and bending moments amplitudes at the center of an anisotropic rectangular plate of uniform thickness and aspect ratio  $b/a = 1/2$ . The first two frequency coefficients for free vibrations of the plate are  $\Omega_1 = 39.3618$ ;  $\Omega_2 = 68.5757$*

$\Omega/\Omega_1$	$W$	$M_x$	$M_y$
0.0	0.0010108	0.018363	0.020055
0.1	0.0010214	0.018579	0.020278
0.2	0.0010546	0.019254	0.020973
0.3	0.0011149	0.020480	0.022235
0.4	0.0012113	0.022446	0.024255
0.5	0.0013618	0.025520	0.027410
0.6	0.0016034	0.030466	0.032477
0.7	0.0020237	0.039087	0.041295
0.8	0.0028866	0.056826	0.059407
0.9	0.0055137	0.110933	0.114574
$(\Omega - \Omega_1)/(\Omega_2 - \Omega_1)$			
0.1	0.0069186	0.145563	0.146589
0.2	0.0033693	0.072447	0.072052
0.3	0.0021922	0.048256	0.047346
0.4	0.0016081	0.036302	0.035095
0.5	0.0012611	0.029247	0.027826
0.6	0.0010325	0.024648	0.023047
0.7	0.0008716	0.021459	0.019693
0.8	0.0007530	0.019160	0.017230
0.9	0.0006627	0.017465	0.015365

TABLE 2

*Non-dimensional displacement and bending moments amplitudes at the center of an anisotropic rectangular plate of uniform thickness and aspect ratio  $b/a = 2/3$ . The first two frequency coefficients for free vibrations of the plate are  $\Omega_1 = 27.2446$ ;  $\Omega_2 = 54.3345$*

$\Omega/\Omega_1$	$W$	$M_x$	$M_y$
0.0	0.0021259	0.029759	0.024805
0.1	0.0021480	0.030102	0.025083
0.2	0.0022173	0.031176	0.025949
0.3	0.0023429	0.033124	0.027521
0.4	0.0025440	0.036244	0.030037
0.5	0.0028577	0.041117	0.033966
0.6	0.0033613	0.048949	0.040274
0.7	0.0042367	0.062582	0.051248
0.8	0.0060334	0.090594	0.073780
0.9	0.0115020	0.175939	0.142391
$(\Omega - \Omega_1)/(\Omega_2 - \Omega_1)$			
0.1	0.0106163	0.169593	0.135226
0.2	0.0051131	0.083734	0.066189
0.3	0.0032934	0.055407	0.043381
0.4	0.0023937	0.041459	0.032122
0.5	0.0018614	0.033260	0.025476
0.6	0.0015123	0.027939	0.021135
0.7	0.0012677	0.024266	0.018109
0.8	0.0010882	0.021632	0.015906
0.9	0.0009522	0.01970	0.014253

TABLE 3

*Non-dimensional displacement and bending moments amplitudes at the center of an anisotropic square plate of uniform thickness  $b/a = 1$ . The first two frequency coefficients for free vibrations of the plate are  $\Omega_1 = 18.2417$ ;  $\Omega_2 = 36.8608$*

$\Omega/\Omega_1$	$W$	$M_x$	$M_y$
0.0	0.0047255	0.053171	0.029462
0.1	0.0047747	0.053771	0.029799
0.2	0.0049286	0.055645	0.030852
0.3	0.0052078	0.059046	0.032762
0.4	0.0056544	0.064491	0.035822
0.5	0.0063515	0.072994	0.040600
0.6	0.0074702	0.086648	0.048276
0.7	0.0094151	0.110406	0.061635
0.8	0.0134062	0.159194	0.089075
0.9	0.0255519	0.307750	0.172645
$(\Omega - \Omega_1)/(\Omega_2 - \Omega_1)$			
0.1	0.0229441	0.285757	0.161305
0.2	0.0110410	0.140197	0.079426
0.3	0.0071058	0.092139	0.052407
0.4	0.0051609	0.068446	0.039098
0.5	0.0040108	0.054488	0.031271
0.6	0.0032570	0.045396	0.026185
0.7	0.0027293	0.039086	0.022667
0.8	0.0023425	0.034520	0.020136
0.9	0.0020496	0.031125	0.018271

TABLE 4

*Non-dimensional displacement and bending moments amplitudes at the center of an anisotropic rectangular plate of uniform thickness and aspect ratio  $b/a = 3/2$ . The first two frequency coefficients for free vibrations of the plate are  $\Omega_1 = 13.8667$ ;  $\Omega_2 = 23.4429$*

$\Omega/\Omega_1$	$W$	$M_x$	$M_y$
0.0	0.0080452	0.082097	0.032965
0.1	0.0081300	0.083014	0.033349
0.2	0.0083949	0.085882	0.034550
0.3	0.0088754	0.091086	0.036731
0.4	0.0096447	0.099421	0.040228
0.5	0.0108459	0.112443	0.045698
0.6	0.0127746	0.133368	0.054499
0.7	0.0161302	0.169800	0.069844
0.8	0.0230207	0.244666	0.101424
0.9	0.0440044	0.472800	0.197774
$(\Omega - \Omega_1)/(\Omega_2 - \Omega_1)$			
0.1	0.0595466	0.653648	0.278519
0.2	0.0290742	0.322315	0.138554
0.3	0.0189634	0.212452	0.092210
0.4	0.0139432	0.157969	0.069285
0.5	0.0109587	0.125642	0.055740
0.6	0.0089922	0.104402	0.046896
0.7	0.0076075	0.089509	0.040754
0.8	0.0065871	0.078601	0.036318
0.9	0.0058106	0.070372	0.033041

TABLE 5

*Non-dimensional displacement and bending moments amplitudes at the center of an anisotropic rectangular plate of uniform thickness and aspect ratio  $b/a = 2$ . The first two frequency coefficients for free vibrations of the plate are  $\Omega_1 = 12.1997$ ;  $\Omega_2 = 18.0591$*

$\Omega/\Omega_1$	$W$	$M_x$	$M_y$
0.0	0.0101820	0.100760	0.035141
0.1	0.0102906	0.101887	0.035551
0.2	0.0106301	0.105411	0.036834
0.3	0.0112463	0.111808	0.039166
0.4	0.0122335	0.122064	0.042910
0.5	0.0137763	0.138102	0.048777
0.6	0.0162565	0.163907	0.058237
0.7	0.0205773	0.208902	0.074775
0.8	0.0294627	0.301515	0.108903
0.9	0.0565633	0.584215	0.213321
$(\Omega - \Omega_1)/(\Omega_2 - \Omega_1)$			
0.1	0.1116861	1.172130	0.436745
0.2	0.0551243	0.581901	0.218522
0.3	0.0363122	0.385686	0.146076
0.4	0.0269493	0.288108	0.110138
0.5	0.0213693	0.230030	0.088831
0.6	0.0176829	0.191736	0.074867
0.7	0.0150811	0.164784	0.065125
0.8	0.0131598	0.144962	0.058052
0.9	0.0116952	0.129940	0.052793

TABLE 6

*Comparison of values of displacement amplitudes and bending moments for a square anisotropic plate of uniform thickness as the number of terms in the Fourier Series approximating function are increased from  $M = N = 10$  (100 terms) to  $M = N = 30$  (900 terms)*

$\Omega/\Omega_1$	Number of terms	$W$	$M_x$	$M_y$
0.0	100	0.00465656	0.0502690	0.0270319
	400	0.00472553	0.0531713	0.0294623
	900	0.00475115	0.0521469	0.0283676
0.3	100	0.00512418	0.0557287	0.0300198
	400	0.00520780	0.0590463	0.0327625
	900	0.00523907	0.0579400	0.0315688
0.5	100	0.00622778	0.0686270	0.0370819
	400	0.00635157	0.0729943	0.0406007
	900	0.00639849	0.0717203	0.0391866
0.7	100	0.00914662	0.1027888	0.0557987
	400	0.00941514	0.1104069	0.0616354
	900	0.00951965	0.1088684	0.0597339
0.9	100	0.02368048	0.2731322	0.1491894
	400	0.02555199	0.3077502	0.1726456
	900	0.02634536	0.3094042	0.1707183

Tables 1 to 5 depict non-dimensional displacement amplitudes and bending moments at the center of anisotropic simply supported rectangular plates of uniform thickness, for several values of the aspect ratios, taking  $M = N = 20$ . The calculations were performed using in all cases 80 bit floating point variables (IEEE-standard temporary reals) in order to ensure accurate results.

Table 6, on the other hand, shows displacement amplitudes and bending moments at the center of an anisotropic square plate as the number of terms is varied from  $M = N = 10$  to  $M = N = 30$ . As this table seems to show, one gets results within 1–2% of real values when  $M = N = 20$ .

The present approach can be extended in a straightforward fashion to the case of plates of non-uniform thickness, presence of orifices, etc.

#### ACKNOWLEDGMENTS

The present study has been sponsored by CONICET Research and Development Program at the Physics Department (UNMDP) and the Institute of Applied Mechanics (Bahía Blanca).

#### REFERENCES

1. A. W. LEISSA 1969 *NASA SP160* Vibration of plates.
2. P. A. A. LAURA and R. DURAN 1975 *Journal of Sound and Vibration* **42**, 129–135. A note on forced vibrations of a clamped rectangular plate.
3. P. A. A. LAURA, J. POMBO and L. E. LUISONI 1976 *Journal of Sound and Vibration* **45**, 225–235. Forced vibrations of circular plates elastically restrained against rotation.
4. E. A. SUSEMIHL and P. A. A. LAURA 1977 *Journal of Ship Research* **21**, 24–29. Forced vibrations of thin, elastic rectangular plates with edges elastically restrained against rotation.
5. S. G. LEKHNITSKII 1968. *Anisotropic Plates*. New York: Gordon and Breach Science Publishers.