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Author's reply[☆]

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The author thanks Drs. Vera and Laura for their comments on the paper [1]. For an isotropic rectangular plate of uniform thickness h , the modulus E and the Poisson ratio ν are constant with respect to time t and the spatial variables x and y . It may be better to put the constant factor before the differential operators and to rewrite Eq. (1) of Ref. [1] as

$$\left[\frac{Eh^3}{12(1-\nu^2)} \right] \nabla^4 w(x, y, t) + \rho h \frac{\partial^2 w(x, y, t)}{\partial t^2} = 0. \quad (1)$$

The mass loaded subdomain as shown in Fig. 1 is $(x_c - d_x/2) < x < (x_c + d_x/2)$ and $(y_c - d_y/2) < y < (y_c + d_y/2)$.

The dynamic equation of the unloaded portion of the plate as mentioned in Ref. [1] is described by Eq. (1) while that of the mass loaded portion is described by Eq. (2) in Ref. [1]. To prevent any confusion, Eq. (2) in Ref. [1] can be rewritten as

$$D \nabla^4 w(x, y, t) + \rho h \frac{\partial^2 w(x, y, t)}{\partial t^2} = 0 \quad \text{for the unloaded portion,} \quad (2a)$$

$$D \nabla^4 w(x, y, t) + \rho h \frac{\partial^2 w(x, y, t)}{\partial t^2} + \frac{\partial^2 (MA'w(x, y, t))}{\partial t^2} = 0 \quad \text{for the loaded portion,} \quad (2b)$$

where $D = Eh^3/12(1-\nu^2)$ is the flexural rigidity of the plate.

Although Eq. (2) of Ref. [1] can be corrected by using the Heaviside's step function in the x and y directions [2] as suggested by Vera and Laura, it seems simpler to express Eq. (2) by two equations as stated above. The above changes should not affect any results presented in Ref. [1] because those numerical results are calculated based on Eq. (7) of Ref. [1].

References

- [1] W.O. Wong, The effects of distributed mass loading on plate vibration behavior, *Journal of Sound and Vibration* (2002) 577–583.
- [2] O. Kopmaz, S. Telli, Free vibrations of a rectangular plate carrying a distributed mass, *Journal of Sound and Vibration* 251 (2002) 39–57.

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